Structural analysis and design -What we do, and what we could do

### Presentation to the Data-centric Engineering Group

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## Structural Engineering vs Mechanical Engineering

- Structural design checks limit states including:
  - Ultimate limit state
  - Serviceability limit state
- The ultimate limit state may be complete collapse, i.e.
  - The displacements are nonlinear
  - The material behaves nonlinearly
- Structural stability is affected by initial geometric imperfections, e.g.
  - Out-of-plumb
  - Out-of-straightness









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## Structural Design – What we do

- Structural design is probabilistically based
- Design criteria are in terms of probability of failure





- Design action  $S^* = S^*(\mathbf{x},t) = S[D(\mathbf{x}), L(\mathbf{x},t), S(\mathbf{x},t), W(\mathbf{x},t), E(\mathbf{x},t), \dots]$
- Capacity  $R = R(\mathbf{x},t) = R[M(\mathbf{x},t), F(\mathbf{x})]$
- S\* and R are random variables
- Generally:  $S^* = S^*(\mathbf{x})$  and  $R = R(\mathbf{x})$

## Structural Design – Actions (loads)

- Loads are expressed in terms of probability of exceedance
- AS/NZS 1170 Structural design actions. Part 0: General principles

Importance level	Annual probability of the design event for safety					
	Wind	Snow	Earthquake			
1	1/100	1/50	1/100			
2	1/500	1/150	1/500			
3	1/1000	1/250	1/1000			
4	1/2000	1/500	1/2500			

#### TABLE 3.2

#### REFERENCE PROBABILITY OF EXCEEDANCE

- Statistics are available for loads (mean, CoV, and distribution)
- E.g. D Normal, L Gamma distribution, W Extreme Type I distribution

## Structural Design – Capacity (Strength)

- The strength of a structure depends on the material (M) and geometric (F) properties
- M: Provides relationship between stress and strain, typically (E,  $f_y$ ,  $f_u$ , ...) and a stress-strain curve + yield surface for combined stress, flow rules, etc.
  - Steel, aluminium, etc (metals):  $M = M(\mathbf{x})$
  - Concrete, timber, etc: M=M(x,t)
- F: Tolerance in fabrication and erection, typically (t, b, imperfections)
- Statistics are available for common M and F, e.g. fy Lognormal;
  E normal; t,b Lognormal etc.

## Structural Design – design criteria

- Probability of failure

$$P_f = P_r[R - S^* \le 0] = P_f[g(R, S^*) \le 0] = \int \dots \int_{g(X) \le 0} f_x(X) dX$$

- X vector of basic random variables, e.g. E,  $f_y$ , D, L, etc.
- $f_{\mathbf{x}}(\mathbf{x})$  joint probability density function (PDF) of basic variable
- g failure function, g(.)  $\leq$  0 implies failure, e.g. Dead and Live load combination:

$$g = R - D - L$$

- Reliability index ( $\beta$ )

$$\beta = \Phi^{-1}(1 - P_f)$$

– Design criterion:

$$P_f \le P_f^0$$
 or  $\beta \ge \beta_0$ 

## Structural Design – design criteria con't

- Practical design uses nominal values of random variable, e.g.
  E<sub>n</sub>, f<sub>y</sub>, D<sub>n</sub>, L<sub>n</sub>, etc
- Design check:

$$\varphi R_n \ge \sum \gamma_i S_{ni}^*$$

- $-\sum \gamma_i S^*_{ni}$  is the load combination,
  - Dead and live:  $\sum \gamma_i S_{ni}^* = 1.2D_n + 1.5L_n$
  - Dead, live and wind:  $\sum \gamma_i S_{ni}^* = 1.2D_n + \psi_c L_n + W_n$
  - Dead live and earthquake:  $\sum \gamma_i S_{ni}^* = D_n + \psi_c L_n + E_n$
- $\phi$  is the resistance factor
- Reliability calibration: Determine  $\varphi$  so that  $\beta \ge \beta_0$  is satisfied
  - Member level
  - System level

## Structural Design – reliability calibration

- Member-based design
  - Build model, apply loads, run analysis (elastic)  $\rightarrow$  M, N, V ...
  - Design check (AS4100), e.g. simple beam:  $\varphi M_p \ge M$
  - Must be satisfied for all members and connections
- System-based design
  - Build model, apply loads (D, L, W...) and introduce a load increment factor (λ), run fully nonlinear analysis (mimic actual behaviour)
  - Design check:  $\varphi_{s}\lambda_{u} \geq 1$







## **Development of system-based design framework**

- DP11 (Rasmussen, Zhang, Ellingwood): Connections assumed not to fail
- DP16 (Rasmussen, Zhang, da Silva): Connection models included in calibration
- DP19 (Rasmussen, Zhang, Khezri, Deierlein): FE Modelling of connections including fracture – all limit states checked







- Finer discretisation, nDOF becoming large
- CPU time is becoming an issue

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- Tall buildings are conventionally designed using expensive wind tunnel tests on scaled models
- Wind tunnel tests predict the pressure distribution on the building
- Simple linear interpolation prove inaccurate in highly nonlinear regions
- Machine learning techniques have proven effective to undertake regression and classification tasks in wind-related applications

Hu, G., Liu, L., Tao, D., Song, J., Kwok, K.C.S., Investigation of wind pressures on tall building under interference effects using machine learning techniques. CoRR abs/1908.07307 (2019) The University of Sydney

#### Constitutive (material) modelling

- Conventionally approach: Condensed experimental data into deterministic laws that are coded in FE software
- Alternative: Use ML to extract stress-strain relationships from experimental or prior data, and implement routines in FE simulations



Gaussian process regression-based constitutive models

Gaussian process regression (GPR) based Constitutive Modelling



## Advantages

- Both underlying relation and uncertainty of data could be captured
- 2. Underlying relation is expressed as a stochastic function transparently
  - 3. No assumption on the model expression is required
  - 4. Suitable for all materials

#### Gaussian process regression-based constitutive models, cont

**Gaussian Process (GP)** define a distribution over a stochastic function.

$$f \sim \mathcal{GP}(m(x), k(x, x'))$$

mean covariance Function values of *n* positions,  $\{f(x_1^*), f(x_2^*), \dots, f(x_i^*), \dots, f(x_n^*)\}$  or  $f(x^*)$ , comply the multivariable Gaussian distribution

$$f(\mathbf{x}^*) \sim \mathcal{N}(\mathbf{m}(\mathbf{x}^*), \mathbf{K}_{**})$$
$$\mathbf{K}_{**} = \begin{bmatrix} k(x_1^*, x_1^*) & k(x_1^*, x_2^*) & \cdots & k(x_1^*, x_n^*) \\ k(x_2^*, x_1^*) & k(x_2^*, x_2^*) & \cdots & k(x_2^*, x_n^*) \\ \vdots & \vdots & \ddots & \vdots \\ k(x_n^*, x_1^*) & k(x_n^*, x_2^*) & \cdots & k(x_n^*, x_n^*) \end{bmatrix}$$



Three samples of the function distributed as the GP with: mean function  $m(x) = 0.25x^2$  and covariance function  $k(x, x') = \exp\left[\frac{-(x-x')^2}{2}\right]$ 

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Gaussian process regression-based constitutive models con't



Loads P1 and P1 –  $\mathcal{N}(15, 1.5)$  kN/m Load W – *Gamma* (20, 7.4) kN

Stress-strain relation

- Data driven stochastic structural analysis is performed using Monte Carlo (MC) simulations (N=3000, Abaqus used)
- Both load uncertainty and material uncertainty are considered in this example •
- The critical value of the deflection at point G is  $R_c = H/500$  (20 mm)

COV	Dataset	Expectation (mm)		Standard deviation (mm)		Probability of failure	
	sıze –	True	GPR	True	GPR	True	GPR
0.05	200		13.51		5.64		13.1%
	400	13.57	13.54	5.67	5.68	12.8%	12.8%
	800		13.55		5.65		12.9%
0.10	200		13.79		6.12		14.7%
	400	13.81	13.84	6.02	6.08	14.6%	14.6%
	800		13.83		6.07		14.7%
0.15	200		14.72		6.17		17.8%
	400	13.94	14.07	6.26	6.21	15.5%	16.2%
	800		14.03		6.22		16.0%

#### Gaussian process regression-based constitutive models con't

- A larger dataset size is required to obtain a good estimation of the expected deflection for data with high uncertainty level
- The probability of failure increases with the uncertainty level increasing
- Without considering the material uncertainty accurately, the probability of failure will be underestimated
- The probability of failure predicted by using the GPR model is conservative and will converge to the reference value with the number of data points increasing

## Deep learning-based method on seismic fragility analysis of bridges considering aging effects

- Seismic fragility: Conditional probability providing the likelihood of a structure (or component) exceeding a predefined level of damage for a given ground motion intensity
- Conventional fragility analysis method requires a series of nonlinear time history analysis (computationally expensive)
- Conventional fragility analysis method is not practical for:
  - time-dependent seismic fragility analysis for deteriorating facilities considering aging effects
  - seismic assessment for a transportation network with many bridges
- For highly repetitive analyses, deep learning models can be good surrogates with high accuracy and efficiency. The mechanism of generating fragility curves using deep learning can be simply regarded as a decision-making process, which compares demand with capacity to classify whether the bridge exceeds the limit state or not.

### Deep learning-based method on seismic fragility analysis of bridges considering aging effects, con't

- Conventional fragility analysis process



# Deep learning-based method on seismic fragility analysis of bridges considering aging effects, con't

- Alternative: For highly repetitive analyses, deep learning models can be good surrogates with high accuracy and efficiency
- Generating fragility curves using deep learning can be regarded as a decision-making process, which compares demand with capacity to determine whether a limit state is exceeded or not
- Problem can be transformed into a binary classification problem



#### Deep learning-based method on seismic fragility analysis of bridges considering aging effects, con't

- Results to date suggest the use of ML proves sufficient accuracy



Fragility curves developed by the DNN-based method and the traditional NLTHA method.

### Error identification in structural design

- Structural members and connections in steel framework tend to be standardised
- Each requires a calculation of strength and deformation response to a structural design code
- Thousands of strength calculations are available in design offices and could be used to train predictive models using ML
- Useful for design or error identification in design



### **Optimised structural design equations**

- Large experimental data sets are available for common types of structural members
- Could be used to train predictive algorithms to optimise efficiency
  - Input parameters: Select input variables that have the most significant effects on the output variable(s), (h, b, d, t, L, f<sub>y</sub>, ...)
  - 2. Choose ML algorithm to generate reasonably simple predictive models
  - 3. Divide datasets is into two subsets (training/testing) model to generate and verify predictive model.



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# Reformulate problems in terms of Bayesian statistics

- Where would this make sense?
  - In problems requiring probabilistic assessment, e.g. reliability analysis
  - In problems with a high degree of uncertainty, e.g. constitutive modelling of materials with high degrees of variability, (timber, 3D printed materials, ...)



R. Ibanez, E. Abisset-Chavanne, Jose Vicente Aguado, David Gonzalez, Elias Cueto, Francisco Chinesta, A Manifold Learning Approach to Data-Driven Computational Elasticity and Inelasticity, Arch Computat Methods Eng (2018) 25:47–57

- In time-dependent problems where data is collected that can inform the model, e.g. structural health monitoring of significant infrastructure (bridges, tunnels, tall buildings, ...)
- In problems where the loading model is associated with significant reliability, e.g. design of wind turbine towers
- There is a field of research on statistical finite element analysis

#### Reformulate problems in terms of Bayesian statistics, con't

- What governing equations lend themselves to this?
  - Dynamic structural analysis (vibrations, earthquake,...)

$$m\frac{\partial^2 u(x,t)}{\partial t^2} + c\frac{\partial u(x,t)}{\partial t} + ku(x,t) = f(x,t)$$

 Transport problems (advection-diffusion-reaction), e.g. carbonation, chloride ingress, hydration in concrete

$$\frac{\partial \phi}{\partial t} + \frac{\partial}{\partial x_i} \left( \beta_i \phi - D_{ij} \frac{\partial \phi}{\partial x_j} \right) + \lambda \phi = Q$$

M. Gharib, M. Khezri, S.J. Foster, Meshless and analytical solutions to the time-dependent advection-diffusion-reaction equation with variable coefficients and boundary conditions, Applied Mathematical Modelling 49 (2017) 220–242
 M. Gharib, M. Khezri, S.J. Foster, A. Castel, Application of the meshless generalised RKPM to the transient advection-diffusion-reaction equation, Computers and Structures, 193 (2017) 172–186

Fluid dynamics in wind and water applications, and fluid-structure interaction applications

$$\frac{\partial \boldsymbol{u}}{\partial t} + (\boldsymbol{u} \cdot \nabla)\boldsymbol{u} - \nu \nabla^2 \boldsymbol{u} = \boldsymbol{g} - \nabla p$$